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SULFATE ATTACK RESISTANCE OF INTERNALLY CURED HIGH-PERFORMANCE CONCRETE CONTAINING MILLET HUSK ASH

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Abstract: Due to the difficulties in maintaining failing concrete structures built with normal concrete (NC), high-performance concrete (HPC), which is extremely dense and has a low water-binder content, has been developed. However, it has been observed that inadequate internal curing in HPC can lead to autogenous shrinkage and micro-cracking. Previous research has indicated that internal curing (IC) agents, such as presaturated lightweight aggregate (LWA) and superabsorbent polymers (SAP), are frequently used to address this issue. It is also noteworthy that the production of HPC necessitates the use of additional supplementary cementitious materials (SCM), particularly silica fume, which is not easily accessible in Nigeria or other Sub-Saharan African countries. This article presents the report of an effort to replace silica fume in HPC with millet husk ash (MHA) as a SCM material. The HPC specimens produced was internally cured with presoaked pumice as LWA and SAP respectively, with the view to establish the effectiveness of MHA for production of HPC. The results of 28, 56 and 90 days for concrete specimen immersed in ordinary water and in 5% of MgSO₄ for C55/67 HPC mixtures for 100 mm concrete cubes having 7.5% silica fume content is compared with MHA based HPC of varied MHA contents of 2.5%, 5%, 7.5%, 10% and 15%). The results reveal 2.5%MHA performed best when compare with the control (i.e 7.5%SF) in resisting 5% MgSO₄ attack with a strength loss factor of 1.89, 1.81 and 1.31% at 28, 56 and 90 days of age and was recommended for use as SCM and IC-agent in HPC.

Keywords: Millet husk ash (MHA); Silica fume (SF); Superabsorbent polymers (SAP); Pre-soaked pumice; and High-performance concrete (HPC)

Introduction

Cement, water, and aggregates are combined to create concrete, a composite material (Neville, 2012; Mudashiru et al., 2021). The foundation of the material is cement which when mixed with water forms a paste that binds aggregates together to form a hard material called concrete. The strength of concrete is commonly considered as the most valuable property because it usually gives an overall picture of the quality of concrete and it is one of the most vital elements in structural design which is specified for compliance purpose (Olawuyi et al., 2020; Mudashiru et al., 2021). Many materials are being added to the concrete mix to improve its properties both in

fresh and hardened states. High-performance concrete (HPC), an alternative to normal concrete (NC), was developed in response to issues with the deterioration and early failure of concrete structures constructed using NC (Mudashiru et al., 2021). Due to its high strength, low water-tobinder ratio (W/B) and high modulus of elasticity, HPC is mostly utilized in the construction industry these days to create tall buildings, construction of tunnels and bridges (Aïtcin, 2004; Orosz, 2017). Thinner structural components can be built with HPC, creating a structure with more aesthetic appeal (Nduka et al., 2020). Buildings with more usable spaces will require less steel to be constructed, and the pressure on the overall structure will be lessened when structural members are built using HPC. Therefore, architects and designers can easily attain greater architectural flexibility, buildable structural shapes and forms, and nearly free reinforcing bars, which results in cheaper labour (Wang et al., 2015). When water is added, and densification occurs in the concrete, the quantity of cement and the added supplementary cementitious materials (SCMs) in HPC mix typically results in a rise in temperature. According to Savva et al. (2018), on the direct relationship between ambient temperature and cementitious materials, the hydrated gel's porosity is increased because the ambient temperature typically affects the cementitious materials' grains This phenomenon causes a fast reaction that obstructs the uniform distribution of hydration products. Because of the combined effects of hydration and pozzolanic reaction, the integration of SCMs induces autogenous shrinkage, chemical shrinkage, and self-desiccation, which has increased the water demand in the concrete (Wu et al., 2017). Using an internal curing method is one of the most efficient and effective ways to address these issues in concrete production. Due to HPC's low permeability, external curing is insufficient to allow water to permeate the concrete, making the IC method one of the most effective and efficient ways to reduce the risk associated with autogenous shrinkage (Olawuyi et al., 2017, Mudashiru et al. 2021). Hence, the incorporation of IC agents which has the ability to absorb water and release it in to the system when the need arises. Studies on the application of several IC agents in the manufacturing of HPC have been conducted; the most widely employed IC agents in the literature are

LWA and superabsorbent polymer (SAP)

(Mudashiru *et al.*, 2021). In order to determine the efficacy of Nigerian supplementary cementitious materials (MHA) and internal curing agent (presoaked pumice) in HPC production, this article will concentrate on the influence of sulphate attack on the compressive strength of millet husk ash (MHA) as an alternative to silica fume (SF) in internally cured high performance concrete.

Materials and Methods Materials

Binders including Portland cement (PC) and SCMs such as SF and MHA, fine and coarse aggregates, superplasticizer, pumice, SAP, magnesium sulphate, and water were the materials used in this study. The Portland cement used in this investigation was Dangote brand 3X (Grade 42.5N), whose characteristics meet BS EN 197-1: 2016 standards. It was purchased in Gidan-Kwano, Minna, Niger State, from a cement store. The MHA that was utilized as SCM was obtained from the burning of the husk in the Concrete Laboratory of the Department of Building, Federal University of Technology, Minna, Niger State, using a locally built incinerator. The burning was done outdoors for roughly twenty-four hours at a temperature of no more than 700 degrees Celsius. After that, the material was allowed to cool before being harvested and ground using a grinding machine. According to ASTM C430-2014, the milled MHA was sieved with a 75µm mesh before being placed in an airtight polythene bag for storage. The SF used as the second SCM for this study was purchased from a Construction Chemical company in Lagos.

Natural sand retained on 300 µm sieve, which is the prerequisite for HPC production, was utilized as the fine aggregate in this study (Shetty, 2004; Neville, 2012; Nduka *et al.*, 2020; Olawuyi *et al.*, 2020). The sieve analysis was used to analyse the physical properties of the sand, such as its specific gravity (SG), fineness modulus (FM),

coefficient of uniformity (Cu), coefficient of curvature (Cc), and dust content. In accordance with typical HPC mixes found in the literature, crushed granite stone that passed through a 13.50 mm sieve size and retained on at least a 9.50 mm sieve size was used as coarse aggregate (Aitcin, 2004; Beushausen and Dehn, 2009; Neville, 2012; Olawuyi and Boshoff, 2018; Nduka *et al.*, 2020; Olawuyi *et al.*, 2021). To reduce the mixture's water requirement, the coarse aggregate was cleaned to get rid of dust particles.

According to Olawuyi and Boshoff (2018), a Superabsorbent polymer (FLOSET 27CS) with a grain size of ≤ 600 μm, manufactured in France by SNF Floerger, was added at 0.2% by weight of binder (bwob). This was done by taking into account that the tea-bag test yielded a SAP absorption capacity of 12 g/grams, which complies with the requirements of the SAP specification for the production of HPC (Olawuyi *et al.*, 2021). The thermoset polymer called SAP is precisely the covalently cross-linked polymers of acrylic acid and acrylamide that are produced by polymerization in bulk solutions and neutralized by alkali hydroxide. Pre-soaked pumice, which was utilized in this investigation, was a porous igneous rock that was crushed to a maximum size of 12.5 mm after being created by explosive volcanic eruptions. Before being used, the pumice was drained after being submerged in water for 24hrs (Olawuyi *et al.*, 2020). According to the specifications of BS EN 1008 (2002), potable water sourced from Federal University of Technology, Minna, Niger State was used for the mixing at 0.3 w/b in this investigation (Ogunbayo *et al.*, 2018). The chemical admixture (superplasticizer) utilized in the typical HPC combinations was sky 504 Masterglenium polymerbased polycarboxylic ether (PCE) superplasticizer from Armorsil Manufacturing Incorporation. It was applied at a concentration of 1.5% by weight of binder as reported by Olawuyi *et al.* (2021).

Methods

Properties of constituent materials

X-ray fluorescence (XRF) was used to analyse the chemical compositions of binders (MHA, SF, and OPC) at the National Geoscience Research Laboratory in Kaduna State and the Laboratory of the Ewekoro Factory of Lafarge Plc. Following the calcination, grinding, and sieving processes, about 100g of these binders were placed in sealed polythene bags and dispatched to determine the oxide compositions in compliance with BS EN 197-1: 2016. Wet sieving was used to ascertain the aggregates samples' particle size distribution. The aggregates and binders' specific gravities were carried out.

Production of HPC specimens

The mix design approach for material proportioning for HPC production was selected based on the work of Aitcin (2004) which was cited by Nduka *et al.* (2020) and Olawuyi *et al.* (2021). The mean goal strength of C55/67 at 28 days was chosen. The following table displays the HPC material proportioning: 0.2% SAP by weight of the binder and 5% pre-soaked pumice by weight of coarse aggregate (bwoca).

Table 1: Materials mix proportioning of the HPC mixtures

Mix proportion	Materials (Kg/m³)								
r	CEM II	SF	MHA	F/A	C/A	PP	SAP	SP	Water
M0a	499.5	40.5		700	997.5		1.08	8.1	156
M0b	499.5	40.5		700	997.5	52.5		8.1	156
M1	526.5		13.5	700	997.5	52.5		8.1	156
M2	513		27	700	997.5	52.5		8.1	156
M3	499.5		40.5	700	997.5	52.5		8.1	156
M4	486		54	700	997.5	52.5		8.1	156

M5 459 81 700 997.5 52.5 8.1 156

NB: CEM= Portland cement, SF= silica fume, MHA= millet husk ash, F/A= fine aggregate, C/A= coarse aggregate, PP= pre-soaked pumice, SAP= superabsorbent polymer, and, SP= superplasticzer

M0=92.5%PC+7.5%SF; M1=97.5%PC+2.5%MHA;

M2=95%PC+5%MHA; M3=92.5%PC+7.5%MHA;

M4=90%PC+10%MHA; M5=85%PC+15%MHA a=0.2% SAP, b=5% pre-soaked pumice

For the SAP internally cured HPCs, the SAP contents (0.2% bwob) were utilised, along with an additional 12.5g/g of water for SAP absorption; for the Pumice internally cured HPCs, the coarse aggregate of the pre-soaked saturated pumice was measured and used at 5% by weight. Prior to compressive strength testing, the cast 100 mm cube HPCs were demoulded and cured for 28, 56 and 90 days by submerging them completely in water and 5% MgSO4 solution (Olawuyi et al., 2021). Results and Discussion

Physical and chemical properties

The results of the XRF analysis of the powder binders (MHA, SF, and PC) are shown in Table 2 below. The total of the major oxides (SiO2 + Al2O3 + Fe2O3) yields 87.20%, which is above the 70% minimum requirement as stated in the ASTMC 618 (2012) standard, indicating that the MHA, is a class N pozzolan. According to ASTM C618 (2012) it is an extremely strong and reactive class F Pozzolan, as shown by the SF main percentage of SiO2 (96.20%). The total ferric oxides (SiO2+Al2O3+Fe2O3), alumina, and silica for the SF (96.91%) exceed the 70% threshold given for the Class of Pozzolan in ASTM C618 (2012). On the other hand, calcium oxide

Table 2: Oxide Composition of Binder Constituents

Oxides	MHA (%)	SF (%)	CEM II (%)
$\overline{\mathrm{SiO}_2}$	71.05	96.20	25.64
Al_2O_3	14.66	0.45	5.24
Fe ₂ O ₃	1.49	0.26	7.15
CaO	1.55	0.05	64.35
MgO	0.73	0.03	0.41
SO_3	0.67	0.10	0.11
K_2O	5.21	0.02	0.05
Na_2O	1.16	0.02	0.31
M2O5	2.06	0.50	0.04
P2O5	1.19	0.4	0.03
LOI	2.10	1.02	0.00
$SiO_2 + Al_2O_3 + Fe_2O_3$	87.20	96.91	38.03

(CaO) makes up the majority of the PC. This conforms to oxides composition for CEM II Portland cement found in literature (Neville, 2012; Mehta & Monteiro, 2014).

The physical characteristics of the aggregate utilized in the investigation are displayed in Figure 1 and Table 3. The fine aggregate's uniformity coefficient (Cu) of 2.39, coefficient of curvature (Cc) of 0.94, and fineness modulus (FM) of 2.88 indicate that it complies with Shetty's (2004) medium sand categorization. The study's coarse aggregates are in the class of uniformly graded stone, with a coefficient of uniformity (Cu) of 1.32 and a coefficient of curvature (Cc) of 0.92. Table 3 shows that coarse and fine aggregates pre-soaked pumice and crushed granite are both suitable for producing HPCs.

Influence of sulphate attack on the compressive strength of the HPCs

Physical/visual assessment, mass loss, residual compressive strength and strength loss factor was used to determine the influence of sulphate attack on the various HPC mix.

Physical / visual assessment

The physical/visual observation of the various HPCs specimen cured in 5% magnesium sulphate (MgSO₄) solution at different curing age is presented in Table 6. At 28, 56 and 90 days, a whitish MgSO₄ crystal and frost precipitation was observed on the surfaces of the concrete cubes for all the HPC mix. A slight deterioration at the

corners and cracks on the surface of the entire specimen was noticeable. The samples containing SAP has fine crack, the pre-soaked pumice incorporated HPC on the other hand was observed to have a very fine and tiny cracks at 28 and 56 days. At 90 days, reduction in cracks formation was observed. The reduction in the crack at 90 days was as a result of internal water reservoir which was provided by the IC-agents which aided further hydration process and as a result of formation of additional C-S-H resulting from the reaction of the binary binders (Olawuyi *et al.*, 2021). None of the HPC specimen was distorted. Formation of white deposit especially in all the specimen's sample may amount to physical deterioration in a longer period other than the period used for this study (Olawuyi *et al.*, 2021).

Table 6: Physical characteristics of internally cured HPCs exposed to sulphate solution

Mix ID	Surface texture	e Size	Colour	Edge	Shape
Physical	characteristics o	f HPC at 28			
days					
M0a	Slightly deterior	rated No change	Whitish deposit	Fine cracks	Perfect cube
M0b	Slightly deterior	rated No change	Whitish deposit	Very fine cracks	Perfect cube
M1	Slightly deterior	rated No change	Whitish deposit	Very fine cracks	Perfect cube
M2	Slightly deterior	rated No change	Whitish deposit	Very fine cracks	Perfect cube
M3	Slightly deterior	rated No change	Whitish deposit	Very fine cracks	Perfect cube
M4	Slightly deterior	rated No change	Whitish deposit	Very fine cracks	Perfect cube
M5	Slightly deterior	rated No change	Whitish deposit	Very fine cracks	Perfect cube
Physical	characteristics o	f HPC at 56			
days					
M0a	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M0b	Smooth	No change	Whitish deposit	Very fine cracks	Perfect cube
M1	Smooth	No change	Whitish deposit	Very fine cracks	Perfect cube
M2	Smooth	No change	Whitish deposit	Very fine cracks	Perfect cube
M3	Smooth	No change	Whitish deposit	Very fine cracks	Perfect cube
M4	Smooth	No change	Whitish deposit	Very fine cracks	Perfect cube
M5	Smooth	No change	Whitish deposit	Very fine cracks	Perfect cube
Physical	characteristics o	f HPC at 90			
days					
M0a	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M0b	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M1	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M2	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M3	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M4	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M5	Smooth	No change	Whitish deposit	Perfect	Perfect cube
M0= 92.5	%PC+7.5%SF: N	M1=97.5%PC+2.5%MHA	A: M2 =95%PC+5%	6MHA: M3 =92.5%	6PC+7.5%MH

b= pre-soaked pumice

14 | Page

a=0.2% SAP 5%:

M4=90%PC+10%MHA: **M5**=85%PC+15%MHA:

Mass loss

Figures 2, 3 and 4 provide results of the mass loss of various HPCs mix cured in MgSO₄ at 28, 56 and 90 days of age. From figure 2, the mass loss values obtained for the controls (M0a and M0b) at 28 days in MgSO₄ solution are: 1.89% and 1.49% respectively while the values for M1, M2, M3, M4 and M5 are; 1.51%, 1.56%, 2.16%, 2.32% and 2.84% respectively.

From Figure 3, the values for mass loss for the controls (M0a and M0b) at 56 days of age are; 1.53%, and 1.24% while for M1, M2, M3, M4 and M5 their mass loss values are; 1.38%, 1.43%, 1.89%, 1.97% and 2.34%. From figure 4, at 90 days of age, the values for mass loss for the controls (M0a and M0b) are; 1.19% and 1.02% while for M1, M2, M3,

M4 and M5 are; 1.18%, 1.24%, 1.54%, 1.72% and 1.97.

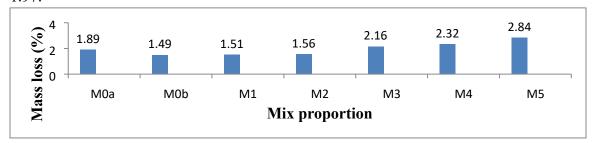


Figure 2: Mass loss of internally cured HPCs subjected to MgSO₄ at 28 days

M0=92.5%PC+7.5%SF: **M1**=97.5%PC+2.5%MHA: **M2**=95%PC+5%MHA: **M3**=92.5%PC+7.5%MHA;

M4=90%PC+10%MHA: **M5**=85%PC+15%MHA: **a**=0.2% SAP: **b**=5% pre-soaked pumice

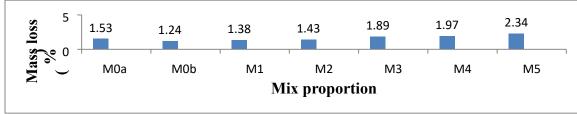


Figure 3: Mass loss of internally cured HPCs subjected to MgSO₄ at 56 days

M0=92.5%PC+7.5%SF: **M1**=97.5%PC+2.5%MHA: **M2**=95%PC+5%MHA: **M3**=92.5%PC+7.5%MHA;

M4=90%PC+10%MHA: **M5**=85%PC+15%MHA: **a**=0.2% SAP: **b**=5% pre-soaked pumice

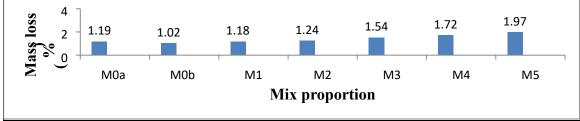


Figure 4: Mass loss of internally cured HPCs subjected to MgSO₄ at 90 days

M0=92.5%PC+7.5%SF; M1=97.5%PC+2.5%MHA; **M2**=95%PC+5%MHA; **M3**=92.5%PC+7.5%MHA;

M4=90%PC+10%MHA; **M5**=85%PC+15%MHA

a=0.2% SAP; **b**=5% pre-soaked pumice

Generally, from the figures, it shows that all the HPCs specimens show similar behaviour of mass losses of the specimen. The phenomenon behind this was as a result of non-absorption of the sulphate solution at 28-, 56- and 90-days immersion in MgSO₄ (Mudashiru *et al.*, 2021). From the figures, it was observed that at 28, 56 and 90 days of age, SF based HPC internally cured with SAP (M0a) suffer more mass loss of 1.89%, 1.53% and 1.19% when compared with SF based HPC internally cured with pre-soaked pumice (M0b) with mass loss value of

1.49%, 1.24% and 1.02%. From the figures, it was observed that SF based HPC internally cured with pre-soaked pumice (M0b) with mass loss value of 1.49%, 1.24% and 1.02% suffer less mass loss when compared with MHA based HPC internally cured with pre-soaked pumice (M1, M2, M3, M4 and M5) with M5 having the highest mass loss of 2.84%, 2.34% and 1.97% at 28, 56 and 90 days of age, also, from the result it was observed that and increases in proportion of MHA lead to an increase in mass loss for all the curing ages with 15% of MHA (M5) having the highest value of mass loss of 2.84%, 2.34% and 1.97% while 2.5% MHA (M1) has the lowest value of mass loss of 1.49%, 1.24% and 1.02% at 28, 56 and 90days.

From the figures, it was also observed that the mass losses of the HPCs decrease at a decrease rate as the curing age increases. The reason behind this may be attributed to additional hydration of C-S-H (Olawuyi et al., 2021) Strength loss factor (SLF)

The effect of MgSO₄ on the compressive strength of the various HPCs was expressed in terms of strength loss factor (SLF) expressed in percentage, and these are illustrated in Figures 5, 6 and 7. From Figure 5, the strength loss factors for the controls (M0a and M0b) cured in MgSO₄ at 28 days is: 1.75% and 1.72% while for M1, M2, M3, M4 and M5 the values are; 1.89%, 2.45%, 2.89%, 2.96% and 3.12%. At 56 days of immersion in sulphate solution, the SLF values from figure 6 for the controls (M0a and M0b) are: 1.73% and 1.69% while for M1, M2, M3, M4 and M5 the SLF are; 1.81%, 2.36%, 2.81%, 2.88% and 3.03%. In figure 7, for the controls (M0a and M0b), the strength SLF at 90 days are: 1.52% and 1.40% while for M1, M2, M3, M4 and M5 the SLF are; 1.31%, 2.06%, 2.43%, 2.58% and 2.63%.

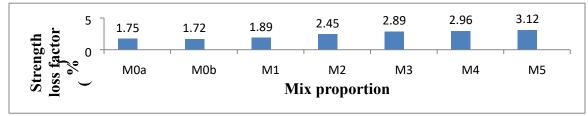


Figure 5: Strength loss factor of internally cured HPCs subjected to MgSO₄ at 28 days **M0**=92.5%PC+7.5%SF; **M1**=97.5%PC+2.5%MHA; **M2**=95%PC+5%MHA;

M3=92.5%PC+7.5%MHA;

M4=90%PC+10%MHA;

M5=85%PC+15%MHA **a**=5% pre-soaked pumice; **b**=0.2% SAP

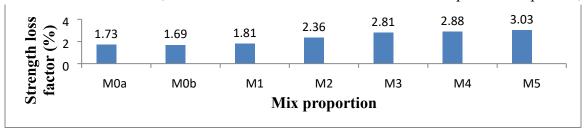
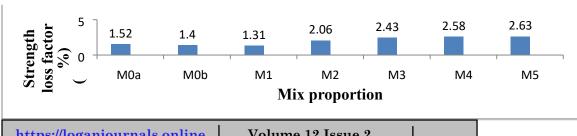


Figure 6: Strength loss factor of internally cured HPCs subjected to MgSO₄ at 56 days

M0=92.5%PC+7.5%SF; **M1**=97.5%PC+2.5%MHA; **M2**=95%PC+5%MHA; **M3**=92.5%PC+7.5%MHA;

M4=90%PC+10%MHA; **M5**=85%PC+15%MHA

a=5% pre-soaked pumice; b=0.2% SAP



16 | Page

Figure 7: Strength loss factor of internally cured HPCs subjected to MgSO₄ at 90 days

M0=92.5%PC+7.5%SF; M1=97.5%PC+2.5%MHA; **M2**=95%PC+5%MHA; **M3**=92.5%PC+7.5%MHA;

M4=90%PC+10%MHA; **M5**=85%PC+15%MHA

a=5% pre-soaked pumice; b=0.2% SAP

From the figures generally, it was observed that all the mixes were affected by MgSO₄ solution. From the figures, at 28, 56 and 90 days of age, SF based HPC internally cured with SAP (M0a) suffer more attack when immersed in sulphate solution with a SLF of 1.75%, 1.73% and 1.52% when compared with SF based HPC internally cured with pre-soaked pumice with SLF of 1.72%, 1.69% and 1.40%. From the figures, it was also observed that as the proportion of MHA increases, SLF increases at a decrease rate from 2.5%MHA to 7.5%MHA and at an increasing rate from 10%MHA to 15%MHA for all the curing ages with 2.5% of MHA internally cured with pre-soaked pumice having the lowest SLF of 1.89%, 1.81% and 1.31% while 15% MHA content has the highest SLF of 3.12%, 3.03% and 2.63% for pre-soaked pumice internally cured HPCs at 28, 56 and 90days

Conclusion

From the study, The SF and MHA used for the study were a good Class F and N Pozzolan with physical and chemical properties that conforms to ASTM C618 (2012) specifications.

Whitish magnesium sulphate crystals, micro-crack pattern and a frosted surface was observed on all the mixes when cured in 5% MgSO₄ solution at all the curing ages with the exception of 28 days where a grey colour with few precipitations was noticed. Curing in 5% MgSO₄ lead to loss in the mass and compressive strength values for all HPCs. But the loss in mass and compressive strength decreased with increase in curing ages. 2.5%MHA performed best when compared with the control (7.5%SF) in resisting MgSO₄ attack and it exhibited the highest residual strength. Hence it has better resistance to chemical attack.

MHA content of 2.5% and 5% pre-soaked pumice are recommended for use as SCM and IC-agent in HPC.

References

Aïtcin, P. C. (2004). High-performance concrete. Taylor & Francis e-Library.

- ASTM C618. (2012). Standard specification for coal fly ash and raw or calcined natural pozzolan for use in concrete. Annual book of ASTM standards.
- ASTM C430. (2014). Standard specification for coal fly ash and raw or calcined natural pozzolan for use in concrete. Annual book of ASTM standards.
- Beushausen, H., & Dehn, F. (2009). High-performance concrete. In Fulton's concrete technology (9th ed.). Cement and Concrete Institute.
- BS EN 1008. (2002). Mixing water for concrete: Specification for sampling, testing, and assessing the suitability of water. British Standards Institution.
- BS EN 197-1. (2016). Composition, specifications, and conformity criteria for common cement. British Standards Institution.
- Mehta, P. K., & Monteiro, J. M. (2014). Concrete: Microstructure, properties, and materials (4th ed.). McGraw-Hill Education.

- Mudashiru, S. A., Olawuyi, B. J., Ayegbokiki, S. T., & Ndayako, S. K. (2021). Influence of magnesium sulphate on compressive strength of rice husk ash-based high-performance concrete. Proceedings of the 3rd School of Environmental Technology International Conference on Sustainable and House Management, 271–278.
- Nduka, D. O., Olawuyi, B. J., Mosaku, T. O., & Joshua, O. (2020). Influence of superabsorbent polymers on properties of high-performance concrete with active supplementary cementitious materials of Nigeria. In W. P. Boshoff et al. (Eds.), RILEM Conference on the Application of Superabsorbent Polymers and Other New Admixtures (SAP 2019, RILEM Bookseries, Vol. 24, pp. 65–74). Springer. https://doi.org/10.1007/978-3-03033342-38
- Neville, A. M. (2012). Properties of concrete (5th ed.). Pearson Education Limited.
- Ogunbayo, B. F., Ajao, A. M., Ogundipe, K. E., Joshua, O., Durotoye, T. O., & Bamigboye, G. O. (2018). Study of aggregate dormancy and its effects on the properties of aggregates and concrete. Open Journal of Civil Engineering, 2(2), 85.
- Olawuyi, B. J., & Boshoff, W. P. (2018). Influence of SAP content and curing age on air void distribution of high-performance concrete using 3D volume analysis. Construction and Building Materials, 135, 580–589.
- Olawuyi, B. J., Saka, R. O., Nduka, D. O., & Babafemi, A. J. (2020). Comparative study of superabsorbent polymers and pre-soaked pumice and internal curing agents in rice husk-based high-performance concrete. In W. P. Boshoff et al. (Eds.), RILEM Conference on the Application of Superabsorbent Polymers and Other New Admixtures (SAP 2019, RILEM Bookseries, Vol. 24, pp. 75–84). Springer.
- Olawuyi, B. J., Babafemi, A. J., & Boshoff, W. P. (2021). Early-age and long-term development of high-performance concrete with SAP. Building and Materials, 267, 121798.
- Orosz, K. (2017). Early age autogenous deformation and cracking of cementitious materials Implications on strengthening of concrete (Unpublished PhD thesis). Luleå Tekniska University, Luleå, Sweden.
- Savva, P., Nicolaides, D., & Petrou, M. F. (2018). Internal curing for mitigating high-temperature concreting effects. Construction and Building Materials, 179, 598–604.
- Shetty, M. S. (2004). Concrete technology: Theory and practice. S. Chand and Company Limited.
- Wang, D., Shi, C., Wu, Z., Xiao, J., Huang, Z., & Fang, Z. (2015). A review on ultra-high-performance concrete: Part II. Hydration, microstructure, and properties. Construction and Building Materials, 96, 368–377.
- Wu, L., Farzadnia, N., Shi, C., Zhang, Z., & Wang, H. (2017). Autogenous shrinkage of high-performance concrete: A review. Construction and Building Materials, 149, 62–75.